## APPENDIX A

## Influent Characteristics

## A1: The BOD concentration

The BOD concentration in the influent varied considerably over 2 years. Summary statistics are shown in Table A1.

Table A1 Summary statistics for inlet BOD (mg/l) to each pond

| Inlet to Pond | Mean | Median | Range |
| :---: | :---: | :---: | :---: |
| Blue | 553 | 446 | $38-2150$ |
| Green | 658 | 565 | $72-1730$ |
| Red | 594 | 483 | $30-1500$ |

Plots of the data indicated that the BOD decreased on average after August 2001. However, this decrease was insignificant compared to the overall large fluctuations. The sample autocorrelation function (acf) plot shown as Figure A1, suggests that the BOD concentration could be adequately described as a completely random time-series (nonseasonal and without trend).

The data were skewed (overall skew $=1.773$ ) and did not approximate to a normal distribution (see Figure A2). A useful transformation was found to be $\log _{10}$. The fitted values after this transformation are shown in Figure A3. The data from each pond showed a similar pattern.

After transformation, the data from each pond was tested to ascertain if the overall mean was the same using analysis of variance $(p=0.484)$. Thus, all inlet BOD data were pooled together; the log-transformed mean was $485 \mathrm{mg} / \mathrm{l}$. All calculations were performed using Genstat 3.2 statistical software.


## LAS

Figure A1: The sample acf plot of the time series of BOD concentration in the inlet


Figure A2: Plots of BOD data in the inlet as fitted to a normal distribution.
(Normal and Half-Normal plots should be straight lines; the histogram should have an even distribution).


Figure A3. Fitted values after a $\log _{10}$ transformation of the BOD data: the fit is improved.

## A2: The suspended solids concentration

The SS concentration in the influent had large fluctuations over the 2 years' of operation. Summary statistics are shown in Table A2.

Table A2 Summary statistics for inlet SS (mg/l) to each pond

| Inlet to Pond | Mean | Median | Range |
| :---: | :---: | :---: | :---: |
| Blue | 1215 | 877 | $76-6016$ |
| Green | 1367 | 945 | $183-6081$ |
| Red | 1181 | 718 | $52-4568$ |

The sample autocorrelation function plot (Figure A4) indicates that the SS concentration in the inlets could be adequately described as a completely random time-series (nonseasonal and without trend).

The data were skewed (overall skew $=1.868$ ) and did not approximate to a normal distribution as shown in Figure A5. Again, a useful transformation was found to be $\log _{10}$. The fitted values after this transformation are shown in Figure A6.

After transformation, an ANOVA test for zero mean difference was performed on the data from each pond $(p=0.406)$. Thus all inlet SS data was pooled together; the grand mean was $1057 \mathrm{mg} / \mathrm{l}$.


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Figure A4. The acf plot for the influent SS concentration over 2 years


Figure A5: Normal plot of all inlet SS concentration values; histogram showing skewed data


Figure A6. Fitted values after a $\log _{10}$ transformation of the $\mathbf{S S}$ data. The fit is improved.

## A3: The ammonia concentration

The ammonia concentration in the influent did not appear to be random (Figure A7). Between August 2000 to October 2001 there appeared to be a seasonal effect (i.e. higher in summer than winter), though this was not evident between November 2001 and June 2002. The autocorrelation function plot highlights the possibility of seasonal fluctuations, see Figure A8. A very simple moving average filter was applied to the data to give local averages (Figure A9). These values were used to calculate ammonia removal efficiency. The fitted values range between $18-40 \mathrm{mg} \mathrm{N} / \mathrm{l}$.


Figure A7. Monthly mean ammonia concentration in the influent: bars show range.


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Figure A8: The acf plot of the ammonia concentration time-series: suggests seasonal effects (negative autocorrelations between lags 4-9 and positive between lags 11-17).


Figure A9: Fitted moving average filter for inlet ammonia concentration

$$
S m\left(x_{t}\right)=0.25 x_{t-1}+0.5 x_{t}+0.25 x_{t+1}
$$

$\boldsymbol{S m}\left(\boldsymbol{x}_{t}\right)=$ smoothed value of $\boldsymbol{x}_{t}$

## A4. The filtered BOD concentration and the supernatant BOD concentration

The filtered BOD concentration and the BOD concentration in the supernatant both followed a random normal distribution and did not require transforming. An ANOVA test for mean difference gave a significance probability of 0.909 for the filtered BOD and 0.828 for the supernatant BOD. All the data were pooled together and the grand mean was $99 \mathrm{mg} / \mathrm{l}$ for filtered BOD and $173 \mathrm{mg} / \mathrm{l}$ for the supernatant BOD.

## A5 The supernatant SS concentration

The SS concentration in the supernatant followed a random normal distribution and did not require transforming. An ANOVA test for mean difference gave a significance probability of 0.613 . All the data were pooled together and the grand mean was 187.5 $\mathrm{mg} / \mathrm{l}$.

## APPENDIX B Duckweed

The family of duckweeds (Lemnaceae) are the smallest flowering plants, and proliferate on still or slow moving water enriched with nutrients. A species of Lemna (Lemna minor) (Figure B1) infested all the pilot-ponds; it originated from the humus tank effluent used to fill the ponds at start-up. Lemna is a real problem for facultative ponds because the tiny fronds float on the surface, forming a dense blanket which effectively blocks out all the light and prevents air penetration. The micro-algae in the water cannot compete for light and die, thus the water beneath the duckweed becomes anaerobic. The weed must be removed (or harvested), otherwise it will eventually settle to the bottom, greatly augmenting the sludge volume and increasing feedback into the water column.


Figure B1: Duckweed (Lemna minor) on the Red pond

The optimum temperature for duckweed growth is between $20-30^{\circ} \mathrm{C}$, but may tolerate temperatures down to $-7^{\circ} \mathrm{C}$. The optimum pH is between 4.5 to 7.5 ; at pH 10 growth is seriously impeded. Wind and waves affect growth by affecting the ability of the fronds to attach to each other and form colonies (Cross, 2002).

The ponds became covered with the duckweed at different times: the Green pond was covered within two weeks of start-up, and the Blue and Red ponds took a month or so to be completely covered. The construction of the outlet meant that the fronds could not flow out. The specific growth rates for duckweed are very high, ranging from 0.1-0.35 $\mathrm{g} / \mathrm{g}$.day, and within two months of operation all the ponds were covered with duckweed mats 3 cm thick. Removal was by manually dragging the weed to one side of the pond using a barrier sign from the site (see Figure B2); ropes were attached to each side of the barrier and two people were required to pull the weed, and another person removed it using a special "trawler" net designed for the purpose (Figure B2a). The ponds were cleared of duckweed in November 2000 (Figures B3 and B4). It reappeared in late May 2001 and May 2002; on each occasion it was quickly removed before it could impact on the ponds.


Figure B2. Duckweed removal in November 2000. Two people are dragging the weed to one side using a barrier sign.


Figure B2a "Trawler" net for the removal of duckweed


Figure B3. Duckweed on the surface of the Red pond just before removal


Figure B4. The surface of the Red pond immediately after removal of the duckweed

The design of the pilot-scale ponds increased the problem of duckweed: it was trapped by the steep sides and the outlet design. A pond with flatter sides would be better as the wind could drive the weed onto the banks where it would be easier to remove. Alternatively, a flexible outlet (which could take water from the surface) would be useful: allowing the weed to flow out before it became established. Duckweeds grow best in water with high concentrations of nutrients, which are inevitable in facultative ponds. However, as they are affected by high pH (>10), a high concentration of actively photosynthesising micro-algae may inhibit growth.

Experiences in France for controlling duckweed, given by CEMAGREF (1997), include: manual removal, using domestic waterfowl or chemicals. Waterfowl are not easy control (unless you clip their wings!) and suitable chemicals are difficult to find because microalgae are also usually sensitive to the same chemicals as duckweed (2,6 dichlorobenonitrile was suggested).

Duckweed ponds may be used as a form of wastewater treatment, and recently work has been carried to compare the performance of these systems with algal based ponds ((Awuah et al., 2001; Smith and Moelyowati, 2001; Calcedo et al., 2002; Zimmo et al.,
2002). Zimmo et al. (2002) found that up to $33 \%$ of the total nitrogen was removed by harvesting the duckweed. Harvesting removes the nutrients from the system, so eliminating sedimentation and recycling from the sludge. The harvested duckweed is high in protein and may be used for animal feed.

## APPENDIX C

## Pond Biology

The identification of the most abundant species in the pond water took place between April 2001 and June 2002. The relative abundance was based on visual or microscopic inspection and the most abundant organism was established by judgement of the relative numbers of individuals. In many cases, more than one species was judged to be dominant. More detailed versions of the figures given in Chapter 5, Section 5.5.4, are given as Figures C1 and C2.

## C1. Algae

The most dominant algal genera in the pilot-scale ponds were Chlamdymonas (Figure C3) and Chlorella. When Chlamdymonas was dominant, usually it was the only dominant organism and turned the ponds a uniform green colour. Of all the algae, a very small species of Chlorella (about $4 \mu \mathrm{~m}$ in diameter) tended to dominate during the winter. Whilst Euglena (Figure C4) and Phacus (Figure C5) were usually present, they were rarely dominant. The turbid waters of the facultative ponds favoured the motile algae, and non-motile, heavier algae such as Scenedesmus only appeared briefly in the Red pond; diatoms such as Navicula and Fragillaria were present in early spring and autumn. The non-motile species of Chlorella which dominated in the winter appeared to be able to remain buoyant in the water.


Figure C1. The dominant organisms identified in effluent samples


Figure C2. The dominant organisms identified in column samples

## C2. Purple bacteria

Only large $(>1 \mu \mathrm{~m})$ bacteria were identified. The most frequently observed was Chromatium (Figure C6), a purple sulphur bacterium. This genus dominated the Blue pond for most of the winter. This is a motile, anaerobic bacterium which stores elemental sulphur in intracellular globules. The most abundant purple bacterium observed in the Red pond was Rhodopseudomonas (Figure C7), (a purple non-sulphur bacterium) which only really dominated after the algae were wiped out by predation effects. At these times, the surface of the water became clear and the purple colouration below became visible. The Green pond was rarely dominated by purple bacteria.

## C3 Other organisms

The other main organisms identified were protozoa (eg. Vorticella and various cilliates), rotifers (Figure C8), and mosquito larvae (Figure C9); though nematodes were also present. The most frequently observed algal predator was Paramecium (Figure C10). This organism bloomed on the surface giving the pond a grey/brown appearance. Being much greater in size than the average alga, the Paramecium could finish off the algae rapidly, before declining themselves.


Figure C3. Chlamdymonas (x 200 BF) Green pond, 19 June 2001


Figure C4 Euglena (x 100 BF) Blue pond 19 June 2001


Figure C5 Phacus (x200 BF) Red pond, 9 July 2001


Figure C6 Chromatium (x 200 BF). Green pond, 3 July 2001


Figure C7 Rhodopseudomonas (x200 BF) Red pond, 12 June 2001


Figure C8 Rotifer (x 100 BF). Red pond. 19 June 2001


Figure C9 Mosquito larvae Red pond, July 2002


Figure C10 Paramecium (x 200 bright field)
Blue pond, 19 June 2001

## APPENDIX D

## Sludge statistics

## D1 Settleable solids concentration in the influent

The concentration of settleable solids in the influent to each pond had a very wide range as shown in Table D1. The very high values correspond to waste sludge from the host works periodically entering the flow. These high values caused the data to be strongly skewed, so a log transformation was required. The ANOVA test for mean difference on the transformed data had a significance probability of $\mathrm{p}=0.525$, thus the data from all the ponds were grouped together. The log transformed (geometric) mean was $32.7 \mathrm{ml} / \mathrm{l}$.

Table D1. Summary statistics of the settleable solid concentration ( $\mathrm{ml} / \mathrm{l}$ ) to all ponds

|  | n | Minimum | Maximum | Mean | Std. Deviation | Skewness |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| BLUE | 54 | 2 | 240 | 43.5 | 52.191 | 2.845 |
| GREEN | 54 | 7 | 225 | 44.7 | 41.799 | 2.186 |
| RED | 53 | 1 | 180 | 42.3 | 42.776 | 1.701 |

## D2 Estimation of the theoretical sludge volume

The estimated cumulative flow to each pond up to each sludge measurement event was based on the average daily flow from the pumps as shown in Table D2. These values were used together with the mean settleable solids concentration to give the theoretical sludge volume originating from the influent as shown in Table D3.

Table D2. Estimation of the total flow to the ponds up to each sludge sampling event

| 1st sampling: 3/10/00 | $\begin{gathered} \text { Daily flow } \\ \left(\mathrm{m}^{3} / \mathrm{d}\right) \end{gathered}$ | Days | $\begin{gathered} \text { Total flow } \\ \left(\mathrm{m}^{3}\right) \end{gathered}$ | Total flow this interval ( $\mathrm{m}^{3}$ ) | Cumulative flow $\left(\mathrm{m}^{3}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Blue | 0.84 | 8 | 6.72 |  |  |
|  | 0.431 | 77 | 33.187 |  |  |
|  | 1.412 | 3 | 4.236 | 44.143 | 44.143 |
| Green | 0.436 | 5 | 2.18 |  |  |
|  | 0.43 | 83 | 35.69 | 37.87 | 37.87 |
| Red | 0.516 | 88 | 45.408 | 45.408 | 45.408 |
| 2nd sampling: 15/3/01 |  |  |  |  |  |
| Blue | 1.412 | 165 | 232.98 | 232.98 | 277.123 |
| Green | 0.43 | 14 | 6.02 |  |  |
|  | 0.801 | 151 | 120.951 | 126.971 | 164.841 |
| Red | 0.516 | 165 | 85.14 | 85.14 | 130.548 |
| 3rd sampling: 2/10/01 |  |  |  |  |  |
| Blue | 1.412 | 8 | 11.296 |  |  |
|  | 0.8937 | 194 | 173.3778 | 184.6738 | 461.7968 |
| Green | 0.801 | 133 | 106.533 |  |  |
|  | 0.567 | 69 | 39.123 | 145.656 | 310.497 |
| Red | 0.516 | 202 | 104.232 | 104.232 | 234.78 |
| 4th sampling:4/2/01 |  |  |  |  |  |
| Blue | 0.79697 | 153 | 121.9364 | 121.9364 | 583.7332 |
| Green | 0.59956 | 153 | 91.73268 | 91.73268 | 402.2297 |
| Red | 0.49732 | 153 | 76.08996 | 76.08996 | 310.87 |

Table D3. Calculation of the theoretical sludge volume originating from the influent

|  | $\qquad$ | Total flow to pond (1) | Total settleable <br> solids <br> $(\mathrm{ml})$ | Total settleable solids $\left(\mathrm{m}^{3}\right)$ |
| :---: | :---: | :---: | :---: | :---: |
| to 2nd October 2000 |  |  |  |  |
| Blue pond | 44.14 | 44143 | 1444800 | 1.44 |
| Green pond | 37.87 | 37870 | 1239485 | 1.24 |
| Red pond | 45.41 | 45408 | 1486203 | 1.49 |
| to 15th March 2001 |  |  |  |  |
| Blue pond | 277.12 | 277123 | 9070235 | 9.07 |
| Green pond | 164.84 | 164841 | 5395245 | 5.40 |
| Red pond | 130.55 | 130548 | 4272836 | 4.27 |
| to 3rd October 2001 |  |  |  |  |
| Blue pond | 461.80 | 461796 | 15114609 | 15.11 |
| Green pond | 310.50 | 310497 | 10162566 | 10.16 |
| Red pond | 234.78 | 234780 | 7684349 | 7.68 |
| to 4th March 2002 |  |  |  |  |
| Blue pond | 583.73 | 583733 | 19105587 | 19.11 |
| Green pond | 402.23 | 402229 | 13164977 | 13.16 |
| Red pond | 310.87 | 310869 | 10174773 | 10.17 |

## D3 Calculation of actual sludge accumulation volume

The base areas were $16.4,15.6$ and $15.4 \mathrm{~m}^{2}$ for the Red, Green and Blue ponds respectively. The bases were roughly rectangular in shape with $90^{\circ}$ sides as shown in Figure 4.3 (Chapter 4) and for sludge measurement were divided into 16 square metre areas as shown in Figure D1. The average sludge volume for each square metre of base was estimated to be numerically equal to the average height of sludge ( m ) measured at its four corner points. The total sludge volume in the pond was taken to be the sum of the 16 volumes.


Figure D1. The pond base showing the sludge height measuring locations (circles). Each square represents $1 \mathrm{~m}^{2}$.

## D4 Calculation of the contribution per person per year

The number of people served by each pond was based on BOD load, assuming each person contributes 50 g BOD per day, as shown in Table D4. These figures were used to calculate the contribution per person per year of sludge volume accumulated.

Table D4. Calculation of the average number of people served by each pond over 20 months

|  | BOD load (g/d) | (A) <br> number of people | (B) number of days | A x B | average no. of people |
| :---: | :---: | :---: | :---: | :---: | :---: |
| BLUE |  |  |  |  |  |
| Phase 1 | 208.8 | 4.2 | 81 | 338.256 | 9.6 |
| Phase 2 | 684.8 | 13.7 | 168 | 2300.928 |  |
| Phase 3 | 473.2 | 9.5 | 113 | 1069.432 |  |
| Phase 4 | 434.6 | 8.7 | 244 | 2120.848 |  |
| total |  |  | 606 | 5829.464 |  |
| GREEN |  |  |  |  |  |
| Phase 1 | 208.7 | 4.2 | 81 | 338.094 | 6.4 |
| Phase 2 | 388.7 | 7.8 | 168 | 1306.032 |  |
| Phase 3 | 388.7 | 7.8 | 113 | 878.462 |  |
| Phase 4 | 274.7 | 5.5 | 244 | 1340.536 |  |
| total |  |  | 606 | 3863.124 |  |
| RED |  |  |  |  |  |
| Phase 1 | 250.5 | 5.0 | 81 | 405.81 | 5.0 |
| Phase 2 | 250.5 | 5.0 | 168 | 841.68 |  |
| Phase 3 | 250.5 | 5.0 | 113 | 566.13 |  |
| Phase 4 | 250.5 | 5.0 | 244 | 1222.44 |  |
| total |  |  | 606 | 3036.06 |  |

## D5. Percentage of volatile solids to total solids in the influent settleable solids.

The concentration of volatile solids (VS) and total solids (TS) was measured regularly on the influent settleable solids. Although the percentage of VS in TS varied widely, between $40-80 \%$, overall there appeared to be an upward trend over the 20 months of operation as shown in Figure D2. This phenomenon remains unaccounted for, though is it most likely due to changes in influent composition over time, or possibly, a change in the performance of the muffle furnace over time. Either way, the change is important as it will have affected either the composition or measurement of the volatile solids in the sludge samples.


Figure D2. The percentage of volatile solids to total solids in the influent

## APPENDIX E POND TEMPERATURE

Between 19 June 2001 and 20 July 2002, temperatures were logged in each pond every hour at six depth intervals from the surface to 1.25 m depth. The measurements were taken using Thermochron iButton (made by Dallas Instruments) as detailed in Section 4.5.3. In total, approximately 170,600 readings were taken. This data has not been included in the main thesis, though the readings from four days with distinct profiles are shown in Figures E1-4. Perhaps the data may be used together with the data from the weather station to create a thermal balance model.




Figure E1 Diurnal temperature profiles for 5 July 2001
High temperatures, stratification. Air temperature: 13.2-26.4 ${ }^{\circ} \mathrm{C}$ Wind speed : 0.4-39. m/s Solar intensity: $242.7 \mathrm{~W} / \mathrm{m}^{2}$ Rainfall $=0.0 \mathrm{~mm}$




Figure E2 Diurnal temperature profiles for 25 October 2001
Medium temperatures; mixing below surface. Air temperature: $7.1-11.4^{\circ} \mathrm{C}$ Wind speed :
$0.1-5.6 . \mathrm{m} / \mathrm{s}$ Solar intensity: $31.4 \mathrm{~W} / \mathrm{m}^{2}$ Rainfall $=1.2 \mathrm{~mm}$




Figure E3 Diurnal temperature profiles for 8 January 2002
Frozen surface, temperature rises with depth, steady all day. Air temperature: $3.9-5.2^{\circ} \mathrm{C}$
Wind speed : $0.1-2.0 \mathrm{~m} / \mathrm{s}$. Solar intensity: $4.3 \mathrm{~W} / \mathrm{m}^{2}$. Rainfall $=0.0 \mathrm{~mm}$




Figure E4 Diurnal temperature profiles for 12 March 2002
Spring mixing: layers not stable relative to each other. Air temperature: $-1.4-10.1^{\circ} \mathrm{C}$.
Wind speed : $0.0-3.8 \mathrm{~m} / \mathrm{s}$ Solar intensity: $85.1 \mathrm{~W} / \mathrm{m}^{2}$. Rainfall $=0.0 \mathrm{~mm}$

